Umwelt (Australia) Pty Ltd

## Proposed Lynwood Quarry Marulan

Report on<br>Soil Survey

0689-1D
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## Attention: Mr John Merrell

Dear Sir,

## PROPOSED LYNWOOD QUARRY, MARULAN REPORT ON SOIL SURVEY

We are pleased to present our report on a soil survey carried out for the above project.

This report documents field and laboratory investigations and provides discussion and recommendations for land capability with respect to existing site conditions and with respect to rehabilitation of areas after quarrying.

Please contact us if you have any questions regarding this report or if you require further assistance.

For and on behalf of

## Asset Geotechnical Engineering Pty Ltd

Mask Baith

## Mark Bartel

BE MEngSc MIEAust CPEng
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## TABLE OF CONTENTS

1.0 INTRODUCTION ..... 2
2.0 SCOPE OF WORK ..... 2
3.0 PROJECT AREA DESCRIPTION \& REGIONAL GEOLOGY ..... 3
4.0 SOIL LANDSCAPES ..... 3
5.0 SUBSURFACE PROFILE FROM TEST PITTING ..... 4
6.0 LABORATORY TEST RESULTS ..... 5
7.0 REHABILITATION ..... 6
INFORMATION SHEETS
APPENDICES
A Field Investigation ResultsB Laboratory Test Results
FIGURES
1 Site Locality
2 Test Locations and Soil Landscape


### 1.0 INTRODUCTION

This report presents the results of a soil survey for the proposed Lynwood hard rock quarry near Marulan. The investigation was commissioned by Mr John Merrell of Umwelt (Australia) Pty Ltd. The work was carried out in accordance with a proposal by Asset Geotechnical Engineering Pty Ltd dated 7 July 2004, reference P0673.

The objective of the survey is to provide information on the surface and subsurface soil conditions as part of an Environmental Impact Statement (EIS) for the proposed quarry. Specifically, the report describes the properties of the soils present in the project area and outlines measures required to achieve a suitable rehabilitation outcome.

This report should be read in conjunction with the attached Information Sheets.

### 2.0 SCOPE OF WORK

The scope of work for the soil survey comprised:

- A review of existing regional maps and reports relevant to the project area, held within our files.
- Review of Soil Landscape Maps prepared by the Department of Infrastructure, Planning and Natural Resources (DIPNR).
- Visual observations of surface features.
- Logging of 22 test pits (TP1 to TP22), to sample and assess the nature and consistency of soils at accessible areas of the project area.
- Carrying out laboratory tests on the recovered soil samples to provide data on chemical and physical properties.
- Engineering assessment and reporting.

The test pits were excavated on $3^{\text {rd }}$ and $4^{\text {th }}$ November 2004 using a rubber-tyred backhoe. The test locations are shown on the attached Figure 2.

The test pits were excavated to depths ranging from 0.35 metres to 1.3 metres depth and were terminated in weathered bedrock. On completion of logging and sampling, each test pit was backfilled with the excavation spoil and lightly tamped using the backhoe bucket before rolling with the backhoe tyre. Remaining spoil was left and trimmed neatly flush or slightly mounded to the adjacent ground surface.

The test pit locations were set out by our engineer and were located by hand-held GPS measurements. The subsurface conditions encountered were recorded during the progress of the excavations. Soil samples were retained for laboratory testing. Surface levels at the test locations were not determined.


Engineering logs are provided in Appendix A to this report. The results of the laboratory testing are summarised in Section 5 and are provided in Appendix B.

### 3.0 PROJECT AREA DESCRIPTION \& REGIONAL GEOLOGY

The project area is located approximately 3 km west of Marulan as shown in the attached Figure 1. The project area occupies approximately 1000 hectares adjacent to the Hume Highway, and is dissected by the Main Southern Railway.

The regional topography includes gently undulating plains, undulating rises, and rolling low hills to steep hills. The overall relief is about 80 m , from about RL 710 m AHD to about RL 630m AHD. The terrain is incised by numerous open depressions and watercourses that generally flow towards Joarimin Creek near the centre of the project area, Marulan Creek in the south, or Lakyersleigh Creek in the northwest. The Main Southern Railway cuts through the project area near the centre running in a roughly westerly direction before crossing Joarimin Creek and heading northwest.

Vegetation includes open forest and woodland that has been cleared over much of the lower areas. Current land use comprises predominantly open grazing.

The 1:250,000 Goulburn Geological Map indicates the project area is underlain predominantly by Bindook Porphyry (quartz feldspar porphyry, dacite, felsite, and tuff) with the southeastern corner underlain by Marulan Granite (granite and granodiorite).

Numerous rocky outcrops and rock covered areas were observed over the elevated hilly parts of the project area.

### 4.0 SOIL LANDSCAPES

The DIPNR soil landscape mapping identifies four landscape units within the project area. The units are summarised below.

## Bindook Road

Undulating low hills on Devonian Bindook Porphyry. This unit is identified by the sub-angular porphyry rock outcrop on upper slopes and crests.

## Bindook Road variant A

This variant features steeper hills and stony ridgelines with more rock outcrop than the Bindook Road landscape.


## Jaqua

This unit is characterised by long foot-slopes and undulating low rises on Devonian Granite and Permian sediments.

## Marulan

Comprising gently undulating rises to undulating low hills formed on Devonian Granite. Distinct surface expression of outcropping well rounded spheroidal Granite tors is common.

The soil landscape boundaries have been transposed from the SCA maps onto the Towrang 1:25,000 Topographic Map as shown in the attached Figure 2.

### 5.0 SUBSURFACE PROFILE FROM TEST PITTING

The generalised subsurface profile as per the test pitting is summarised below:

| Layer | Description | Depth to Base (m) |
| :--- | :--- | ---: |
| Topsoil | SILT / Sandy SILT / Silty SAND, low plasticity fines, fine to medium sand | $0.05 / 0.20$ |
| Slopewash | Clayey SAND / Sandy SILT / Gravelly SILT, fine to medium grained, low plasticity <br> fines | $0.15 / 0.4$ |
| Residual | SAND / Clayey SAND, fine to medium grained | $0.4 / 1.15$ |
| Residual | CLAY / Sandy CLAY, medium plasticity | $0.4 / 1.0$ |
| Bedrock | PORPHYRY or GRANITE, medium to coarse grained | -- |

### 6.0 LABORATORY TEST RESULTS

Results from the laboratory testing undertaken on selected soil samples are included in Appendix B and are summarised in Table 1 below.

| Sample | Engineering Description | 픈 |  |  | $\stackrel{E}{\sim}$ | 은 <br> 읃 <br> ㅇ |  | $\begin{aligned} & \text { y } \\ & \text { © } \\ & \text { © } \\ & \text { ㄷ } \\ & \frac{H}{\omega} \\ & \text { E } \end{aligned}$ |  |  | $\begin{aligned} & \text { O } \\ & \text { O } \\ & \hline \text { O } \\ & \hline \end{aligned}$ |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| TP1 / 0-0.1 | Sandy SILT (Topsoil) | 5.5 | 0.090 | 10 | 0.9 | 15 |  |  |  |  |  |  |  |  |
| TP1 / 0.05-0.15 | Gravelly Clayey SAND (Slopewash) | 6.0 | 0.090 | 14 | 1.3 | 33 | 3.1 | 6 | 0.3 | >100 | 43 |  |  |  |
| TP1 / 0.5-0.6 | Sandy CLAY (Residual) | 5.1 | 0.080 | 8 | 0.6 | 62 | 6.2 | 5 | 3.8 | 63 | 35 |  |  |  |
| TP2 / 0-0.1 | Sandy SILT (Topsoil) | 4.7 | 0.240 | 10 | 2.4 | 38 |  |  |  |  |  |  |  |  |
| TP2 / 0.2-0.3 | Silty SAND (Slopewash) | 5.3 | 0.080 | 14 | 1.1 | 25 |  |  |  |  |  |  |  |  |
| TP2 / 0.4-0.5 | Sandy CLAY (Residual) | 5.0 | 0.110 | 8 | 0.9 | 47 |  |  |  |  |  |  |  |  |
| TP3 / 0.05-0.15 | Sandy SILT (Topsoil) | 7.1 | 0.210 | 10 | 2.1 | 35 | 7.1 | 6 | 0.4 | 51 | 19 |  |  |  |
| TP3 / 0.25-0.35 | Clayey SAND (Slopewash) | 7.1 | 0.070 | 14 | 1.0 | 22 | 3.0 | 5 | 0.6 | 75 | 17 |  |  |  |
| TP4 / 0-0.1 | Silty SAND (Topsoil) | 5.2 | 0.030 | 14 | 0.4 | 5 | 3.1 | 8 |  |  |  | 68 | 135 | 0.021 |
| TP4 / 0.3-0.4 | Clayey SAND (Residual) | 5.4 | 0.015 | 14 | 0.2 | <5 | 2.5 | 6 |  |  |  | 42 | 78 | 0.007 |
| TP5 / 0.05-0.15 | Sandy SILT (Topsoil) | 4.5 | 0.075 | 10 | 0.8 | 60 |  |  |  |  |  |  |  |  |
| TP6 / 0-0.15 | SILT (Topsoil) | 4.8 | 0.070 | 10 | 0.7 | 15 |  |  |  |  |  |  |  |  |
| TP7 / 0-0.15 | Sandy SILT (Topsoil) | 4.8 | 0.100 | 10 | 1.0 | 30 |  |  |  |  |  |  |  |  |
| TP8 / 0-0.1 | Silty SAND (Topsoil) | 5.4 | 0.100 | 14 | 1.4 | 8 | 5.8 | 8 |  |  |  | 260 | 180 | 0.029 |
| TP8 / 0.4-0.6 | SAND (Residual) | 5.4 | 0.030 | 17 | 0.5 | 8 |  |  |  |  |  |  |  |  |
| TP9 / 0-0.1 | SILT (Topsoil) | 4.5 | 0.230 | 10 | 2.3 | 49 |  |  |  |  |  |  |  |  |
| TP10 / 0-0.05 | SILT (Topsoil) | 4.5 | 0.100 | 10 | 1.0 | 14 | 5.6 | 6 |  |  |  | 390 | 86 | 0.038 |
| TP10 / 0.05-0.15 | Sandy SILT (Slopewash) | 4.5 | 0.090 | 10 | 0.9 | 7 |  |  |  |  |  |  |  |  |
| TP10 / 0.5-0.6 | Sandy CLAY (Residual) | 5.2 | 0.070 | 8 | 0.6 | 64 | 6.9 | 5 |  |  |  | 200 | 27 | 0.02 |
| TP11 / 0-0.1 | Sandy SILT (Topsoil) | 4.6 | 0.330 | 10 | 3.3 | 45 |  |  |  |  |  |  |  |  |
| TP13/0-0.05 | SILT (Topsoil) | 4.7 | 0.180 | 10 | 1.8 | 8 |  |  |  |  |  |  |  |  |
| TP13/0.1-0.2 | Sandy SILT (Residual) | 4.6 | 0.070 | 10 | 0.7 | 32 |  |  |  |  |  |  |  |  |
| TP14 / 0-0.1 | Silty SAND (Topsoil) | 5.8 | 0.080 | 14 | 1.1 | 12 | 8.3 | 6 |  |  |  | 260 | 135 | 0.032 |
| TP14 / 0.1-0.2 | Gravelly Silty SAND (Residual) | 5.4 | 0.055 | 14 | 0.8 | 29 |  |  |  |  |  |  |  |  |
| TP14 / 0.4-0.6 | Sandy CLAY (Residual) | 5.3 | 0.045 | 8 | 0.4 | 26 | 10.4 | 5 |  |  |  | 67 | 82 | 0.009 |
| TP16 / 0-0.1 | Silty SAND (Topsoil) | 5.0 | 0.050 | 14 | 0.7 | 10 |  |  |  |  |  |  |  |  |
| TP18/0.05-0.1 | Silty SAND (Topsoil) | 5.3 | 0.040 | 14 | 0.6 | 10 | 3.8 | 6 |  |  |  | 120 | 185 | 0.017 |
| TP18/0.15-0.25 | Clayey SAND (Slopewash) | 4.8 | 0.040 | 14 | 0.6 | 18 |  |  |  |  |  |  |  |  |
| TP18/0.6-0.7 | Sandy CLAY (Residual) | 5.5 | 0.060 | 8 | 0.5 | 35 |  |  |  |  |  |  |  |  |
| TP19 / 0.05-0.1 | SILT (Topsoil) | 5.3 | 0.090 | 10 | 0.9 | 110 | 3.3 | 6 |  |  |  | 200 | 50 | 0.012 |
| TP19 / 0.5-0.65 | CLAY (Residual) | 5.1 | 0.120 | 8 | 1.0 | 120 | 15.8 | 8 |  |  |  | 84 | 55 | 0.013 |
| TP21 / 0.05-0.1 | Silty SAND (Topsoil) | 5.0 | 0.095 | 14 | 1.3 | 79 |  |  |  |  |  |  |  |  |
| TP22 / 0-0.1 | SILT (Topsoil) | 4.8 | 0.150 | 10 | 1.5 | 150 |  |  |  |  |  |  |  |  |



The laboratory testing indicates that the soils are not dispersive by nature, are assessed to be slightly to moderately acidic and non-saline to slightly saline. The soils generally have a low Cation Exchange Capacity (CEC), which will limit the soil's ability to retain nutrients. The soil phosphorous levels are generally sufficient for grazing / open pasture use. However, the potassium and sulphur levels are relatively low and should be improved during rehabilitation to facilitate plant growth should it be desired to establish permanent pasture.

### 7.0 REHABILITATION

Topsoil from areas to be quarried should be stripped and stockpiled separately from lower subsoils. During rehabilitation after quarrying, the topsoil should be spread and amended as follows, based on the proposed land use:

## Areas to be used for Native Vegetation (i.e. not grazing or cultivation)

- No amendment is considered necessary.


## Areas to be used for Permanent Grazing Land:

- Increase pH to a target value of between 6 and 8.5. To raise the pH by 1, lime should be spread at the rate of $2500 \mathrm{~kg} / \mathrm{ha}$ and thoroughly mixed to 100 mm depth.
- Potassium should be raised to a target minimum value of $125 \mathrm{mg} / \mathrm{kg}$. A dosage rate of $200 \mathrm{~kg} / \mathrm{ha}$ is recommended for initial treatment.
- Organic materials (e.g. manure), ammonium sulphur, or gypsum should be added to improve sulphur levels. A dosage rate of 10 to 20 kg of sulphur per hectare should be applied.


## * * * *

## SCOPE OF SERVICES

The geotechnical report ("the report") has been prepared in accordance with the scope of services as set out in the contract, or as otherwise agreed, between the Client and Asset Geotechnical Engineering Pty Ltd ("Asset"). The scope of work may have been limited by a range of factors such as time, budget, access and/or site disturbance constraints.

## RELIANCE ON DATA

Asset has relied on data provided by the Client and other individuals and organizations, to prepare the report. Such data may include surveys, analyses, designs, maps and plans. Asset has not verified the accuracy or completeness of the data except as stated in the report. To the extent that the statements, opinions, facts, information, conclusions and/or recommendations ("conclusions") are based in whole or part on the data, Asset will not be liable in relation to incorrect conclusions should any data, information or condition be incorrect or have been concealed, withheld, misrepresented or otherwise not fully disclosed to Asset.

## GEOTECHNICAL ENGINEERING

Geotechnical engineering is based extensively on judgment and opinion. It is far less exact than other engineering disciplines. Geotechnical engineering reports are prepared for a specific client, for a specific project and to meet specific needs, and may not be adequate for other clients or other purposes (e.g. a report prepared for a consulting civil engineer may not be adequate for a construction contractor). The report should not be used for other than its intended purpose without seeking additional geotechnical advice. Also, unless further geotechnical advice is obtained, the report cannot be used where the nature and/or details of the proposed development are changed.

## LIMITATIONS OF SITE INVESTIGATION

The investigation programme undertaken is a professional estimate of the scope of investigation required to provide a general profile of subsurface conditions. The data derived from the site investigation programme and subsequent laboratory testing are extrapolated across the site to form an inferred geological model, and an engineering opinion is rendered about overall subsurface conditions and their likely behaviour with regard to the proposed development. Despite investigation, the actual conditions at the site might differ from those inferred to exist, since no subsurface exploration program, no matter how comprehensive, can reveal all subsurface details and anomalies.
The engineering logs are the subjective interpretation of subsurface conditions at a particular location and time, made by trained personnel. The actual interface between materials may be more gradual or abrupt than a report indicates.

## SUBSURFACE CONDITIONS ARE TIME DEPENDENT

Subsurface conditions can be modified by changing natural forces or man-made influences. The report is based on conditions that existed at the time of subsurface exploration. Construction operations adjacent to the site, and natural events such as floods, or ground water fluctuations, may also affect subsurface conditions, and thus the continuing adequacy of a geotechnical report. Asset should be kept appraised of any such events, and should be consulted to determine if any additional tests are necessary.

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Where ground conditions encountered at the site differ significantly from those anticipated in the report, either due to natural variability of subsurface conditions or construction activities, it is a condition of the report that Asset be notified of any variations and be provided with an opportunity to review the recommendations of this report. Recognition of change of soil and rock conditions requires experience and it is recommended that a suitably experienced geotechnical engineer be engaged to visit the site with sufficient frequency to detect if conditions have changed significantly.

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## OTHER LIMITATIONS

Asset will not be liable to update or revise the report to take into account any events or emergent circumstances or fact occurring or becoming apparent after the date of the report.

## LOG ABBREVIATIONS AND NOTES

| METHOD borehole logs |  | excavation logs |  |
| :---: | :---: | :---: | :---: |
| AS | auger screw * | NE | natural excavation |
| AD | auger drill * | HE | hand excavation |
| RR | roller / tricone | BH | backhoe bucket |
| W | washbore | EX | excavator bucket |
| CT | cable tool | DZ | dozer blade |
| HA | hand auger | R | ripper tooth |
| D | diatube |  |  |
| B | blade / blank bit |  |  |
| V | $\checkmark$-bit |  |  |
| T | TC-bit |  |  |
| * bit shown by suffix e.g. ADV |  |  |  |
| coring |  |  |  |
| NMLC, NQ, PQ, HQ |  |  |  |
| SUPPORT |  |  |  |
| bore | le logs | exc | ion logs |
| N | nil |  | nil |
|  | mud |  | shoring |
|  | casing | B | benched |
| NQ | NQ rods |  |  |
| CORE-LIFT |  |  |  |
|  | casing installed |  |  |
| barrel withdrawn |  |  |  |
| NOTES, SAMPLES, TESTS |  |  |  |
|  | disturbed |  |  |
| B | bulk disturbed |  |  |
| U50 | thin-walled sample, 50 mm diameter |  |  |
| HP | hand penetrometer (kPa) |  |  |
| SV | shear vane test (kPa) |  |  |
| DCP | dynamic cone penetrometer (blows per 100 mm penetration) |  |  |
| SPT | standard penetration test |  |  |
| N* | SPT value (blows per 300mm) |  |  |
| Nc | SPT with solid cone |  |  |
|  | refusal of DCP or SPT |  |  |

## USCS SYMBOLS

GW Well graded gravels and gravel-sand mixtures, little or no fines.
GP Poorly graded gravels and gravel-sand mixtures, little or no fines.
GC Silty gravels, gravel-sand-silt mixtures.
GC Clayey gravels, gravel-sand-clay mixtures.
SW Well graded sands and gravelly sands, little or no fines.
Poorly graded sands and gravelly sands, little or no fines.
Silty sand, sand-silt mixtures.
Clayey sand, sand-clay mixtures
Inorganic silts of low plasticity, very fine sands, rock flour, silty or clayey fine sands.
CL Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays.
OL Organic silts and organic silty clays of low plasticity.
$\mathrm{MH} \quad$ Inorganic silts of high plasticity.
$\mathrm{CH} \quad$ Inorganic clays of high plasticity.
$\mathrm{OH} \quad$ Organic clays of medium to high plasticity.
PT Peat muck and other highly organic soils.


## GRAPHIC LOG



Boundaries

- known _-_-_- probable ......................... possible


## WEATHERING

| STRENGTH |  |
| :--- | :--- |
| EL | extremely low |
| VL | very low |
| L | low |
| M | medium |
| $H$ | high |
| VH | very high |
| EH | extremely high |

RQD (\%)
$=$ sum of intact core pieces $>2 \times$ diameter $\times 100$ total length of section being evaluated

## DEFECTS

| type |  | coating |  |
| :--- | :--- | :--- | :--- |
| JT | joint | cl | clean |
| PT | parting | st | stained |
| SZ | shear zone | ve | veneer |
| SM | seam | co | coating |
|  |  |  |  |
| shape |  | roughness |  |
| pl | planar | po | polished |
| cu | curved | sl | slickensided |
| un | undulating | sm | smooth |
| st | stepped | ro | rough |
| ir | irregular | vr | very rough |

inclination
measured above axis and perpendicular to core

ASSET GEOTECHNICAL
geotechnical engineering consultants

## AS1726-1993

Soils and rock are described in the following terms, which are broadly in accordance with AS1726-1993.

## SOIL

## MOISTURE CONDITION

## Term Description

Dry Looks and feels dry. Cohesive and cemented soils are hard, friable or powdery. Uncemented granular soils run freely through the hand.
Moist Feels cool and darkened in colour. Cohesive soils can be moulded. Granular soils tend to cohere.
Wet As for moist, but with free water forming on hands when handled. Moisture content of cohesive soils may also be described in relation to plastic limit $\left(W_{P}\right)$ or liquid limit $\left(W_{L}\right)$ [>> much greater than, > greater than, < less than, $\ll$ much less than].

## CONSISTENCY OF COHESIVE SOILS

| Term | Su (kPa) | Term | Su (kPa) |
| :--- | :--- | :--- | :--- |
| Very soft | $<12$ | Very Stiff | $100-200$ |
| Soft | $12-25$ | Hard | $>200$ |
| Firm | $25-50$ | Friable | - |
| Stiff | $50-100$ |  |  |
|  |  |  |  |
| DENSITY OF GRANULAR SOILS |  |  |  |
| Term | Density Index(\%) | Term | Density Index (\%) |
| Very Loose | $<15$ | Dense | $65-85$ |
| Loose | $15-35$ | Very Dense | $>85$ |
| Medium Dense | $35-65$ |  |  |
|  |  |  |  |
| PARTICLE SIZE |  | Size (mm) |  |
| Name | Subdivision | $>200$ |  |
| Boulders |  | $63-200$ |  |
| Cobbles |  | $20-63$ |  |
| Gravel | coarse | $6-20$ |  |
|  | medium | $2.36-6$ |  |
| Sand | fine | $0.6-2.36$ |  |
|  | coarse | $0.2-0.6$ |  |
| Silt \& Clay | fine |  | $0.075-0.2$ |
|  |  | $<0.075$ |  |

## MINOR COMPONENTS

| Term | Proportion by Mass <br> coarse grained | fine grained |
| :--- | :--- | :--- |
|  | $\leq 5 \%$ | $\leq 15 \%$ |
| Trace | $5-2 \%$ | $15-30 \%$ |
| Some |  |  |

## SOIL ZONING

## ROCK

| SEDIMENTARY ROCK TYPE DEFINITIONS |  |  |  |
| :---: | :---: | :---: | :---: |
| Rock Type D | Definition (more than 50\% of rock consists of .....) |  |  |
| Conglomerate .. | ... gravel sized ( $>2 \mathrm{~mm}$ ) fragments. |  |  |
| Sandstone .. | ... sand sized (0.06 to 2mm) grains. |  |  |
| Siltstone .. | ... silt sized ( $<0.06 \mathrm{~mm}$ ) particles, rock is not laminated. |  |  |
| Claystone .. | ... clay, rock is not laminated. |  |  |
| Shale .. | ... silt or clay sized particles, rock is laminated. |  |  |
| LAYERING |  |  |  |
| Term | Description |  |  |
| Massive | No layering apparent. |  |  |
| Poorly Developed | Layering just visible. Little effect on properties. |  |  |
| Well Developed | Layering distinct. Rock breaks more easily parallel to layering. |  |  |
| STRUCTURE |  |  |  |
| Term | Spacing (mm) | Term | Spacing |
| Thinly laminated | <6 | Medium bedded | 200-600 |
| Laminated | 6-20 | Thickly bedded | 600-2,000 |
| Very thinly bedded | d 20-60 | Very thickly bedded | > 2,000 |
| Thinly bedded | 60-200 |  |  |
| STRENGTH |  |  |  |
| Term Is | Is50 (MPa) | Term Is | 0 (MPa) |
| Extremely Low < | <0.03 | High 1.0 | - 3.0 |
| Very low 0 | 0.03-0.1 | Very High 3.0 | - 10.0 |
| Low 0 | 0.1-0.3 | Extremely High > | 0.0 |
| Medium 0 | 0.3-1.0 |  |  |
|  | NOTE: Is50 = Point Load Strength Index |  |  |
| WEATHERING |  |  |  |
| Term D | Description |  |  |
| Residual Soil S | Soil derived from weathering of rock; the mass structure and substance fabric are no longer evident. |  |  |
| Extremely ..... | Rock is weathered to the extent that it has soil properties (either disintegrates or can be remoulded). Fabric of original rock is still visible. |  |  |
| Highly .... R | Rock strength usually highly changed by weathering; rock may be highly discoloured. |  |  |
| Moderately ..... R | Rock strength usually moderately changed by weathering; rock may be moderately discoloured. |  |  |
| Slightly .... R | Rock is slightly discoloured but shows little or no change of strength from fresh rock. |  |  |
| Fresh R | Rock shows no signs of decomposition or staining. |  |  |

## DEFECT DESCRIPTION

Type
may be patchy
Coating $\quad$ Visible coating $\leq 1 \mathrm{~mm}$ thick. Thicker soil material de-
No visible coating or discolouring scribed as seam.

Layers Continuous exposures.
Lenses Discontinuous layers of lenticular shape.
Pockets Irregular inclusions of different material.

## SOIL CEMENTING

$\begin{array}{ll}\text { Weakly } & \text { Easily broken up by hand. } \\ \text { Moderately } & \text { Effort is required to break up the soil by hand. }\end{array}$

## USCS SYMBOLS <br> Symbol Description

GW Well graded gravels and gravel-sand mixtures, little or no fines.
GP Poorly graded gravels and gravel-sand mixtures, little or no fines.
GM Silty gravels, gravel-sand-silt mixtures.
GC Clayey gravels, gravel-sand-clay mixtures
SW Well graded sands and gravelly sands, little or no fines. SP Poorly graded sands and gravelly sands, little or no Poorly
fines.
SM Silty sand, sand-silt mixtures.
SC Clayey sand, sand-clay mixtures.
ML
Inorganic silts of low plasticity, very fine sands, rock flour, silty or clayey fine sands.
Inorganic clays of low to medium plasticity, gravelly
clays, sandy clays, silty clays.
Organic silts and organic silty clays of low plasticity. Inorganic silts of high plasticity.
Inorganic clays of high plasticity.
Organic clays of medium to high plasticity.
$\begin{array}{ll}\mathrm{CH} & \text { Inorganic clays of high plasticity. } \\ \text { OH } & \text { Organic clays of medium to high plasticity. } \\ \text { PT } & \text { Peat muck and other highly organic soils. }\end{array}$
Joint
Parting

Sheared Zone

Seam

Shape
Planar
Curved
Undulating
Stepped
Irregular
Roughness
Polished
Slickensided
Smooth
Rough
Very Rough $\quad$ Many large surface irregularities, amplitude generally $>1 \mathrm{~mm}$. Feels like very coarse sandpaper.

Coating
Clean
A surface or crack across which the rock has little or no tensile strength. May be open or closed. A surface or crack across which the rock has little or no tensile strength. Parallel or sub-parallel to layering/ bedding. May be open or closed.
Zone of rock substance with roughly parallel, near planar, curved or undulating boundaries cut by closely spaced joints, sheared surfaces or other defects. Seam with deposited soil (infill), extremely weathered insitu rock (XW), or disoriented usually angular fragments of the host rock (crushed).

Consistent orientation.
Gradual change in orientation.
Wavy surface.
One or more well defined steps.
Many sharp changes in orientation.

Shiny smooth surface.
Grooved or striated surface, usually polished.
Smooth to touch. Few or no surface irregularities Many small surface irregularities (amplitude generally
$\mathrm{MH} \quad$ norganic silts of high plasticity

|  | Home | Previous | Next | TOC | 11 May 2005 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 0689-1D |  |  |  |  |  |

## APPENDIX A

Field Investigation Results

Excavation Log assetgeo@ callaustralia.net.au

| EX no: | TP01 |
| :--- | :--- |
| sheet: | 1 of 1 |
| job no.: | $0689-1$ |


| client: | UMWELT (AUSTRALIA) PTY LTD | started: | 3.11 .2004 |
| :--- | :--- | :--- | :--- |
| principal: | READYMIX | finished: | 3.11 .2004 |
| project: | PROPOSED HARD ROCK QUARRY | logged: | MAB |
| location: | MARULAN | checked: | MAB |
| equipment: | 4WD BACKHOE | RL surface: |  |
| dimensions: | $0.45 m$ wide by 2.0m long | datum: |  |
| excavation information | material information |  |  |



| PO Box 3385Rouse Hill NSW 2155Ph: (022) 98362144Fax: (02) 98360225assetgeo@ callaustralia.net.au | EX no: | TP02 |
| :---: | :---: | :---: |
|  | sheet: | 1 of 1 |
|  |  |  |
|  | job no.: | 0689-1 |

Excavation Log assetgeo@ callaustralia.net.au

| client: | UMWELT (AUSTRALIA) PTY LTD | started: | 3.11 .2004 |
| :--- | :--- | :--- | :--- |
| principal: | READYMIX | finished: | 3.11 .2004 |
| project: | PROPOSED HARD ROCK QUARRY | logged: | MAB |
| location: | MARULAN | checked: | MAB |
| equipment: | 4WD BACKHOE | RL surface: |  |
| dimensions: | $0.45 m$ wide by 2.0 m long | datum: |  |



## Excavation Log

| EX no: | TP03 |
| :--- | :--- |
| sheet: | 1 of 1 |
| job no.: | $0689-1$ |


| client: | UMWELT (AUSTRALIA) PTY LTD | started: |  |
| :--- | :--- | :--- | :--- |
| principal: | READYMIX | finished: | 3.11 .2004 |
| project: | PROPOSED HARD ROCK QUARRY | logged: | MAB |
| location: | MARULAN | checked: | MAB |
| equipment: | 4WD BACKHOE | RL surface: |  |
| dimensions: | $0.45 m$ wide by 2.0 m long | datum: |  |



Excavation Log

| client: | UMWELT (AUSTRALIA) PTY LTD | started: | 3.11 .2004 |
| :--- | :--- | :--- | :--- |
| principal: | READYMIX | finished: | 3.11 .2004 |
| project: | PROPOSED HARD ROCK QUARRY | logged: | MAB |
| location: | MARULAN | checked: | MAB |
| equipment: | 4WD BACKHOE | RL surface: |  |
| dimensions: | 0.45 m wide by 2.0 m long | datum: |  |
| excavation information | material information |  |  |



## Excavation Log



## Excavation Log



## Excavation Log



Excavation Log

| EX no: | TP08 |
| :--- | :--- |
| sheet: | 1 of 1 |
| job no.: | $0689-1$ |


| client: | UMWELT (AUSTRALIA) PTY LTD | started: | 3.11 .2004 |
| :--- | :--- | :--- | :--- |
| principal: | READYMIX | finished: | 3.11 .2004 |
| project: | PROPOSED HARD ROCK QUARRY | logged: | MAB |
| location: | MARULAN | checked: | MAB |
| equipment: | 4WD BACKHOE | RL surface: |  |
| dimensions: | $0.45 m$ wide by 2.0 m long | datum: |  |
| excavation information | matial information |  |  |



Excavation Log

| client: | UMWELT (AUSTRALIA) PTY LTD | started: | 3.11 .2004 |
| :--- | :--- | :--- | :--- |
| principal: | READYMIX | finished: | 3.11 .2004 |
| project: | PROPOSED HARD ROCK QUARRY | logged: | MAB |
| location: | MARULAN | checked: | MAB |
| equipment: | 4WD BACKHOE | RL surface: |  |
| dimensions: | 0.45 m wide by 2.0 m long | datum: |  |
| excavation information | material information |  |  |



## Excavation Log



| PO Box 3385 | EX no: | TP 11 |
| :---: | :---: | :---: |
| Rouse Hill NSW 2155 |  |  |
| Ph: (02) 98362144 | sheet: | 1 of 1 |
| Fax: (02) 98360225 |  |  |
| geo@ callaustralia.net.au | job no.: | 0689-1 |

Excavation Log

| client: | UMWELT (AUSTRALIA) PTY LTD | started: | 3.11 .2004 |
| :--- | :--- | :--- | :--- |
| principal: | READYMIX | finished: | 3.11 .2004 |
| project: | PROPOSED HARD ROCK QUARRY | logged: | MAB |
| location: | MARULAN | checked: | MAB |
| equipment: | 4WD BACKHOE | RL surface: |  |
| dimensions: | 0.45 m wide by 2.0 m long | datum: |  |
| excavation information | material information |  |  |



Excavation Log

| EX no: | TP 12 |
| :--- | :--- |
| sheet: | 1 of 1 |
| job no.: | $0689-1$ |


| client: | UMWELT (AUSTRALIA) PTY LTD | started: | 3.11 .2004 |
| :--- | :--- | :--- | :--- |
| principal: | READYMIX | finished: | 3.11 .2004 |
| project: | PROPOSED HARD ROCK QUARRY | logged: | MAB |
| location: | MARULAN | checked: | MAB |
| equipment: | 4WD BACKHOE | RL surface: |  |
| dimensions: | 0.45 m wide by 2.0 m long | datum: |  |
| excavation information | material information |  |  |



| PO Box 3385 | EX no: | TP 13 |
| :---: | :---: | :---: |
| Rouse Hill NSW 2155 |  |  |
| Ph: (02) 98362144 | sheet: | 1 of 1 |
| Fax: (02) 98360225 <br> geo@callaustralia.net.au | job no.: | 0689-1 |

Excavation Log

| client: | UMWELT (AUSTRALIA) PTY LTD | started: | 3.11 .2004 |
| :--- | :--- | :--- | :--- |
| principal: | READYMIX | finished: | 3.11 .2004 |
| project: | PROPOSED HARD ROCK QUARRY | logged: | MAB |
| location: | MARULAN | checked: | MAB |
| equipment: | 4WD BACKHOE | RL surface: |  |
| dimensions: | 0.45 m wide by 2.0 m long | datum: |  |
| excavation information | material information |  |  |



| PO Box 3385 | EX no: | TP 14 |
| :---: | :---: | :---: |
| Rouse Hill NSW 2155 |  |  |
| Ph: (02) 98362144 | sheet: | 1 of 1 |
| Fax: (02) 98360225 |  |  |
| geo@ callaustralia.net.au | job no.: | 0689-1 |

Excavation Log assetgeo@callaustralia.net.au

## client:

UMWELT (AUSTRALIA) PTY LTD
started:
4.12.2004
$\begin{array}{ll}\text { principal: } & \text { READYMIX } \\ \text { project: } & \text { PROPOSED HARD ROCK QUARRY }\end{array}$
finished: 4.12.2004
project:
MARULAN
logged:
MAB

| location: | MARULAN |
| :--- | :--- |
| equipment: | 4WD BACKHOE |

RL su
datum:

| excavation information |  | material information |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |



| PO Box 3385 | EX no: | TP 15 |
| :---: | :---: | :---: |
| Rouse Hill NSW 2155 |  |  |
| Ph: (02) 98362144 | sheet: | 1 of 1 |
| Fax: (02) 98360225 |  |  |
| geo@ callaustralia.net.au | job no.: | 0689-1 |

Excavation Log assetgeo@callaustralia.net.au

| client: |
| :--- |
| princt |
| proje |

Excavation Log

| EX no: | TP16 |
| :--- | :--- |
| sheet: | 1 of 1 |
| job no.: | $0689-1$ |


| client: | UMWELT (AUSTRALIA) PTY LTD | started: | 4.12 .2004 |
| :--- | :--- | :--- | :--- |
| principal: | READYMIX | finished: | 4.12 .2004 |
| project: | PROPOSED HARD ROCK QUARRY | logged: | MAB |
| location: | MARULAN | checked: | MAB |
| equipment: | 4WD BACKHOE | RL surface: |  |
| dimensions: | $0.45 m$ wide by 2.0m long | datum: |  |
| excavation information | material information |  |  |



Excavation Log

| EX no: | TP17 |
| :--- | :--- |
| sheet: | 1 of 1 |
| job no.: | $0689-1$ |


| client: | UMWELT (AUSTRALIA) PTY LTD | started: | 4.12 .2004 |
| :--- | :--- | :--- | :--- |
| principal: | READYMIX | finished: | 4.12 .2004 |
| project: | PROPOSED HARD ROCK QUARRY | logged: | MAB |
| location: | MARULAN | checked: | MAB |
| equipment: | 4WD BACKHOE | RL surface: |  |
| dimensions: | $0.45 m$ wide by 2.0m long | datum: |  |
| excavation information | material information |  |  |



## Excavation Log



| PO Box 3385 | EX no: | TP 19 |
| ---: | :--- | :--- |
| Rouse Hill NSW 2155 |  |  |
| Ph: (02) 9836 2144 | sheet: | 1 of 1 |
| Fax: (02) 9836 0225 |  |  |
| geo@ callaustralia.net.au | job no.: | $0689-1$ |
|  |  |  |

Excavation Log assetgeo@callaustralia.net.au

| client: | UMWELT (AUSTRALIA) PTY LTD | started: | 4.12 .2004 |
| :--- | :--- | :--- | :--- |
| principal: | READYMIX | finished: | 4.12 .2004 |
| project: | PROPOSED HARD ROCK QUARRY | logged: | MAB |
| location: | MARULAN | checked: | MAB |
| equipment: | 4WD BACKHOE | RL surface: |  |
| dimensions: | 0.45 m wide by 2.0 m long | datum: |  |
| excavation information | material information |  |  |



Excavation Log


Excavation Log

| EX no: | TP21 |
| :--- | :--- |
| sheet: | 1 of 1 |
| job no.: | $0689-1$ |


| client: | UMWELT (AUSTRALIA) PTY LTD | started: | 4.12 .2004 |
| :--- | :--- | :--- | :--- |
| principal: | READYMIX | finished: | 4.12 .2004 |
| project: | PROPOSED HARD ROCK QUARRY | logged: | MAB |
| location: | MARULAN | checked: | MAB |
| equipment: | 4WD BACKHOE | RL surface: |  |
| dimensions: | $0.45 m$ wide by 2.0 m long | datum: |  |
| excavation information | matial information |  |  |



| PO Box 3385 | EX no: | TP22 |
| ---: | :--- | :--- |
| Rouse Hill NSW 2155 |  |  |
| Ph: (02) 9836 2144 |  |  |
| Fax: (02) 9836 0225 | sheet: | 1 of 1 |
| geo@ callaustralia.net.au | job no.: | $0689-1$ |
|  |  |  |

Excavation Log

| client: | UMWELT (AUSTRALIA) PTY LTD | started: | 4.12 .2004 |
| :--- | :--- | :--- | :--- |
| principal: | READYMIX | finished: | 4.12 .2004 |
| project: | PROPOSED HARD ROCK QUARRY | logged: | MAB |
| location: | MARULAN | checked: | MAB |
| equipment: | 4WD BACKHOE | RL surface: |  |
| dimensions: | 0.45 m wide by 2.0 m long | datum: |  |
| excavation information | material information |  |  |




## APPENDIX B

Laboratory Test Results

```
Office:
PO BOX 48
ERMINGTON NSW 2115
Laboratory:
1/4 ABBOTT ROAD
SEVEN HILLS NSW 2147
Telephone: (02) 98388903
Fax: (02) 98388919
A.C.N. 003614695
A.B.N. \(\quad 81829182852\)
```


## ANALYTICAL REPORT for:

ASSET GEOTECHNICAL ENGINEERING PTY LTD
15 SANDLEWOOD CLOSE
ROUSE HILL 2155
ATTN: MARK BARTEL

JOB NO:
SAL15321
CLIENT ORDER: 0689-1
DATE RECEIVED: 15/11/04
DATE COMPLETED: 26/11/04
TYPE OF SAMPLES: SOILS
NO OF SAMPLES: 29

NATA Accredited Laboratory
Number: 1884

NATA ENDORSED TEST REPORT
This document shall not be reproduced,

|  | SAMPLES | $\begin{array}{r} \mathrm{pH} \\ 1: 5 \end{array}$ | $\begin{array}{r} \text { COND } \\ \mathrm{dS} / \mathrm{m} \end{array}$ | $\begin{array}{r} \mathrm{Cl} \\ \mathrm{mg} / \mathrm{kg} \end{array}$ | $\begin{array}{r} \mathrm{CEC} \\ \mathrm{cmol}+/ \mathrm{kg} \end{array}$ | * EMERS. <br> Class |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | TP1/0-0.1 | 5.5 | 0.090 | 15 |  |  |
| 2 | TP2/0-0.1 | 4.7 | 0.24 | 38 |  |  |
| 3 | TP2/0.2-0.3 | 5.3 | 0.080 | 25 |  |  |
| 4 | TP2/0.4-0.5 | 5.0 | 0.11 | 47 |  |  |
| 5 | TP4/0-0.1 | 5.2 | 0.030 | 5 | 3.1 | 8 |
| 6 | TP4/0.3-0.4 | 5.4 | 0.015 | <5 | 2.5 | 6 |
| 7 | TP5/0.05-0.15 | 4.5 | 0.075 | 60 |  |  |
| 8 | TP6/0-0.15 | 4.8 | 0.070 | 15 |  |  |
| 9 | TP7/0-0.05 | 4.8 | 0.10 | 30 |  |  |
| 10 | TP8/0-0.1 | 5.4 | 0.10 | 8 | 5.8 | 8 |
| 11 | TP8/0.4-0.6 | 5.4 | 0.030 | 8 |  |  |
| 12 | TP9/0-0.1 | 4.5 | 0.23 | 49 |  |  |
| 13 | TP10/0-0.05 | 4.5 | 0.10 | 14 | 5.6 | 6 |
| 14 | TP10/0.05-0.15 | 4.5 | 0.090 | 7 |  |  |
| 15 | TP10/0.5-0.6 | 5.2 | 0.070 | 64 | 6.9 | 5 |
| 16 | TP11/0-0.1 | 4.6 | 0.33 | 45 |  |  |
| 17 | TP13/0-0.05 | 4.7 | 0.18 | 8 |  |  |
| 18 | TP13/0.1-0.2 | 4.6 | 0.070 | 32 |  |  |
| 19 | TP14/0-0.1 | 5.8 | 0.080 | 12 | 8.3 | 6 |
| 20 | TP14/0.1-0.2 | 5.4 | 0.055 | 29 |  |  |
| 21 | TP14/0.4-0.6 | 5.3 | 0.045 | 26 | 10.4 | 5 |
| 22 | TP16/0-0.1 | 5.0 | 0.050 | 10 |  |  |
| 23 | TP18/0.05-0.1 | 5.3 | 0.040 | 10 | 3.8 | 6 |
| 24 | TP18/0.15-0.25 | 4.8 | 0.040 | 18 |  |  |
| 25 | TP18/0.6-0.7 | 5.5 | 0.060 | 35 |  |  |
| 26 | TP19/0.05-0.1 | 5.3 | 0.090 | 110 | 3.3 | 6 |
| 27 | TP19/0.5-0.65 | 5.1 | 0.12 | 120 | 15.8 | 8 |
| 28 | TP21/0.05-0.1 | 5.0 | 0.095 | 79 |  |  |
| 29 | TP22/0-0.1 | 4.8 | 0.15 | 150 |  |  |
| DUPLICATES: |  |  |  |  |  |  |
| 19 | TP14/0-0.1 | 5.8 | 0.090 | 11 | 8.1 | 6 |
| MDL |  | 0.1 | 0.001 | 5 | 0.1 |  |
| Meth | od Code | WA1 | WA2 | WA 4 | S7 | C43 |
| Prep | aration | P5 | P5 | P5 | P5 | P1 |

## SYDNEY <br> ANALYTICALIES

JOB NO: SAL15321
CLIENT ORDER: 0689-1

## SAMPLES

Total P $\mathrm{mg} / \mathrm{kg}$
$\begin{array}{rr}* \text { Av.K } \\ \mathrm{mg} / \mathrm{kg} & * \text { Tot.S } \\ \%\end{array}$

## 5 TP4/0-0.1

6 TP4/0.3-0.4
10 TP8/0-0.1

| 135 | 0.021 |
| ---: | ---: |
| 78 | 0.007 |
| 180 | 0.029 |
| 86 | 0.038 |
| 27 | 0.020 |
| 135 | 0.032 |
| 82 | 0.009 |
| 185 | 0.017 |
| 50 | 0.012 |
| 55 | 0.013 |
| 130 | 0.029 |


| MDL | 5 | 1 | 0.002 |
| :--- | ---: | ---: | ---: |
| Method Code | WA15 | S4 | HT3 |
| Preparation | P5 | P5 | P5 |
|  |  |  |  |

## LABORATORY DUPLICATE REPORT

JOB NO: SAL15321
CLIENT ORDER: 0689-1

| Sample <br> Number | Analyte | Units | MDL | Sample <br> Result | Duplicate Result | \%RPD |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| TP14/0-0.1 | pH |  | 0.1 | 5.8 | 5.8 | 0 |
| TP14/0-0.1 | Conductivity | dS/m | 0.001 | 0.080 | 0.090 | 12 |
| TP14/0-0.1 | Chloride | $\mathrm{mg} / \mathrm{kg}$ | 5 | 12 | 11 | 8 |
| TP14/0-0.1 | CEC | cmol+/kg | 0.1 | 8.3 | 8.1 | 2 |
| TP14/0-0.1 | *Emerson Class | Class |  | 6 | 6 | 0 |
| TP14/0-0.1 | Total P | $\mathrm{mg} / \mathrm{kg}$ | 5 | 260 | 230 | 12 |
| TP14/0-0.1 | Available K | $\mathrm{mg} / \mathrm{kg}$ | 1 | 135 | 130 | 4 |
| TP14/0-0.1 | *Total Sulphur | - | 0.002 | 0.032 | 0.029 | 10 |

Acceptance criteria:
RPD <50\% for low level ( $<20 x M D L$ )
$\operatorname{RPD}<30 \%$ for medium level ( $20-100 \times M D L$ )
RPD <15\% for high level (>100xMDL)
No limit applies at <2xMDL
MDL $=$ Method Detection Limit
All results are within the acceptance criteria

## S Y D N E Y <br> ANALYTICAL <br> LABORATORIES

Page 5 of 5

## ANALYTICAL REPORT

JOB NO: SAL15321
CLIENT ORDER: 0689-1

## METHODS OF PREPARATION AND ANALYSIS

The tests contained in this report have been carried out on the samples as received by the laboratory.

P5 Sample dried, split and crushed to -150um
P1 Analysis performed on sample as received
WA1 $\mathrm{pH}-1: 5$ soil/water extract
Determined by APHA 4500B
WA2 Conductivity - 1:5 soil/water extract
Determined by APHA 2510B
WA4 Chloride - 1:5 soil/water extract
Determined by APHA 4110B
S7 Cation Exchange Capacity \& Exchangeable/Soluble Cations Determined by Silver Thiourea Method CEC-1
*C43 Modified Emerson Crumb Test: Based on AS1547-1990 Appendix F
WA15 Total Phosphorus - H2SO4/HF Digestion Determined by APHA 4500BF
*S4 Available Phosphorus - Bray Extract ( 0.03 N NH4F) Determined by APHA 4500F
*HT3 Total Sulphur - Determined by High Temperature Furnace

The laboratory's NATA registration does not cover performance of this service
A preliminary report was faxed on 26/11/04

Laboratory:
1/4 ABBOTT ROAD
SEVEN HILLS NSW 2147
Telephone: (02) 98388903
Fax:
(02) 98388919
A.C.N. 003614695
A.B.N. $\quad 81829182852$

ANALYTICAL REPORT for:

ASSET GEOTECHNICAL ENGINEERING PTY LTD
15 SANDLEWOOD CLOSE
ROUSE HILL 2155
ATTN: MARK BARTEL

JOB NO:
CLIENT ORDER: 0698-1
DATE RECEIVED: 15/11/04
DATE COMPLETED: 26/11/04
TYPE OF SAMPLES: SOILS
NO OF SAMPLES: 4

## S Y D N E Y <br> 

## ANALYTICAL REPORT

JOB NO: SAL15321B CLIENT ORDER: 0698-1

|  | SAMPLES |
| :--- | :--- |
| 1 | $\mathrm{TP} 1 / 0.05-0.15$ |
| 2 | $\mathrm{TP} 1 / 0.5-0.6$ |
| 3 | $\mathrm{TP} 3 / 0.05-0.15$ |
| 4 | $\mathrm{TP} 3 / 0.25-0.35$ |

MDL
Method Code Preparation

6.0
5.1
7.1
7.1
$\begin{array}{rr}0.1 & 0.001 \\ \text { C1 } & \text { WA2 } \\ \text { P5 } & \text { P5 }\end{array}$
COND.
$\mathrm{dS} / \mathrm{m}$

0.090
0.080
0.21
0.070

CEC
cmol $+/ \mathrm{kg}$
3.1
6.2
7.1
3.0
0.1

S7
P5

ESP
\%
0.3
3.8
0.4
0.6
0.1

C35
P5
*Resis. ohm.m
$>100$
63
51 75

## SYDNEY <br> ANALYTICAL <br> LABORATORIES

JOB NO: SAL15321B CLIENT ORDER: 0698-1

## SAMPLES

* EMERS.

Cl
SO4 $\mathrm{mg} / \mathrm{kg}$

43
1 TP1/0.05-0.15
2 TP1/0.5-0.6
6
5
33
35
3 TP3/0.05-0.15
6
62
4 TP3/0.25-0.35
5
22
17

| MDL |  | 5 | 5 |
| :--- | ---: | ---: | ---: |
| Method Code | C43 | WA4 | WA6 |
| Preparation | P1 | P5 | P5 |
| RESULTS ON DRY BASIS |  |  |  |

## ANALYTICAL REPORT

JOB NO: SAL15321B
CLIENT ORDER: 0698-1

## METHODS OF PREPARATION AND ANALYSIS

The tests contained in this report have been carried out on the samples as received by the laboratory.

P5 Sample dried, split and crushed to -150um
P8 Sample dried and crushed to pass 6.7 mm sieve
P1 Analysis performed on sample as received
C1 pH - AS1289.4.3.1
WA2 Conductivity - 1:5 soil/water extract
Determined by APHA 2510B
S7 Cation Exchange Capacity \& Exchangeable/Soluble Cations Determined by Silver Thiourea Method CEC-1
C35 Exchangeable Sodium Percentage - Silver Thiourea Extract Determined by APHA 3500B
*C21 Electrical Resistivity - RTA T185
*C43 Modified Emerson Crumb Test: Based on AS1547-1990 Appendix F
WA4 Chloride - 1:5 soil/water extract Determined by APHA 4110B
WA6 Sulphate - 1:5 soil/water extract Determined by APHA 4110B

The laboratory's NATA registration does not cover performance of this service
A preliminary report was faxed on 26/11/04

```
Office:
PO BOX 48
ERMINGTON NSW 2115
Laboratory:
1/4 ABBOTT ROAD
SEVEN HILLS NSW 2147
Telephone: (02) 98388903
Fax:
(02) 98388919
A.C.N. 003614695
A.B.N. \(\quad 81829182852\)
```


## ANALYTICAL REPORT for:

ASSET GEOTECHNICAL ENGINEERING PTY LTD
15 SANDLEWOOD CLOSE
ROUSE HILL 2155
ATTN: MARK BARTEL
JOB NO: SAL15321C
CLIENT ORDER: 0704-1
DATE RECEIVED: 18/11/04
DATE COMPLETED: 30/11/04
TYPE OF SAMPLES: SOIL
NO OF SAMPLES: 1

NATA Accredited Laboratory
Number 1884愙
NATA ENDORSED TEST REPORT This document shall not be reproduced. except in full.


Issued on 09/12/04
Lance Smith
(Chief Chemist)

## SYDNEY <br> ANALYTICAL <br> LABORATORIES

## ANALYTICAL REPORT

## JOB NO: SAL15321C

 CLIENT ORDER: 0704-1| SAMPLES | $\begin{gathered} \mathrm{pH} \\ 1: 5 \end{gathered}$ | $\begin{gathered} \text { COND. } \\ \mathrm{dS} / \mathrm{m} \end{gathered}$ | $\begin{array}{r} \text { CEC } \\ \mathrm{cmol}+/ \mathrm{kg} \end{array}$ | $\begin{array}{r} \text { ESP } \\ \% \end{array}$ | $\begin{array}{r} \text { *P SORP } \\ \mathrm{mg} / \mathrm{kg} \end{array}$ | *EMERS. Class |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $1 \mathrm{Ex} 1 / 0.1-0.5$ | 4.7 | 0.025 | 4.6 | 1.1 | 580 | 6 |

0.1

C1
P5
0.001

WA2
P5
0.1

S7
P5
0.1

C35
P5

1
S9
P5

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## ANALYTICAL REPORT

JOB NO: SAL15321C CLIENT ORDER: 0704-1

## METHODS OF PREPARATION AND ANALYSIS

The tests contained in this report have been carried out on the samples as received by the laboratory.

P5 Sample dried, split and crushed to -150um
P1 Analysis performed on sample as received
C1 pH - AS1289.4.3.1
WA2 Conductivity - 1:5 soil/water extract Determined by APHA 2510B
S7 Cation Exchange Capacity \& Exchangeable/Soluble Cations Determined by Silver Thiourea Method CEC-1
C35 Exchangeable Sodium Percentage - Silver Thiourea Extract Determined by APHA 3500B
*S9 Phosphorus Sorption - Dept of Agriculture Standard Method Determined by APHA 4500F
*C43 Modified Emerson Crumb Test: Based on AS1547-1990 Appendix F

The laboratory's NATA registration does not cover performance of this service
A preliminary report was faxed on 30/11/04
0689-10 Home Previous Next TOC 11 May 2005

FIGURES



## APPENDIX 4B

## Soil Landscape Description

## Appendix 4B-Soil Landscape Description

## Soil landscape descriptions are adapted from DIPNR (2003)

## Bindook Road Variation A

Bindook Road variation A is the predominate soil landscape unit in the area north of the Main Southern Railway. The crests and upper slopes of Bindook Road variation A soils are characterised by Paralithic Bleached Leptic Tenosols (Lithosols) with the mid and upper slopes being characterised by Brown Kurosols (Red and Yellow Podzolic Soils). Lower slopes of the soil landscape unit are characterised by Grey Sodosols (Solodic Soils).

The Bindook Road variant A soil landscape typically contains three horizons. The topsoil consists of two horizons (A1 and A2), with the materials comprising the A1 horizon ranging from a weak sandy loam to a silty/fine granular clay loam. pH for the A1 horizon range from 4.5 to 5.5 . The A2 horizon is comprised of bleached dilatant sandy clay loam. The pH of the A2 horizon ranges from 4.5 to 5.5 with the texture of the material ranging from sandy clay loam to silty clay loam.

The A1 and A2 horizons of the Bindook Road variant A soil landscape overly a sub-angular medium clay subsoil being reddish brown to yellowish brown in colour. The pH of the subsoil ranges from 5 to 6 with the structure ranging from moderate to strong pedality.

## Bindook Road

Bindook Road is the predominate soil landscape unit to the south of the Main Southern Railway. This soil landscape unit also runs along the western edge of the project area, with patches also found in the northeast of the project area as shown on Figure 5.2 of the main text of the Environmental Impact Statement (EIS).

The Bindook Road soil landscape typically has four horizons, being the A1 and A2 horizons as well as the B2 and B3 horizons. The A1 horizon ranges from a weak sandy loam to a silty/fine sandy granular clay loam. Field pH ranges from 4.5 to 5.5 . The structure of the A 1 horizon ranges from massive to moderate pedality for the silty/fine sandy loam and from massive to weak pedal for the weak sandy loam. Fragment sizes range from coarse gravel to cobbles with the peds being $2-5 \mathrm{~mm}$ and angular in shape.

The A2 horizon is comprised of a bleached dilatant sandy clay loam with a pH range of 4.5 to 5.5 . The fragment sizes of coarse gravel to cobbles overlies the strong brown sub angular blocky medium clay of the subsoil B2 horizon. The B2 horizon has a pH range of 5 to 6 with fragment sizes ranging from coarse gravel to cobbles. Ped sizes for the B2 horizon are in the order of $20-50 \mathrm{~mm}$. The B2 horizon is characterised by moderate to strong pedality with fragment sizes ranging from coarse gravel to cobbles.

The greyish brown medium heavy sandy clay of the $\mathrm{B} 2 / 3$ horizon underlies the A 2 horizon. The sandy clay material is characterised by angular to sub-angular blocky ped shapes ranging from gravel to cobbles. This horizon is highly erodible, dispersive and sodic in nature.

## Jaqua

The Jaqua soil landscape unit is found in the project area along the main channel of Joarimin Creek. The Jaqua soil landscape unit is also located in the southeastern corner of the project area. The rises of the Jaqua soil landscape unit are characterised by Yellow Kurosols (yellow podsolic soils) with the foot slopes being characterised by yellow and brown Sodosols (Soloths, Solidic Soils and Solodized Solonetz). The channels are characterised by Stratic Rudosols comprising alluvial soils.

The Jaqua soil landscape unit is generally composed of A1, A2 and B2 horizons. The A1 and A2 horizon is comprised of a number of materials typically being poorly structured loamy sand to sandy clay loam and a bleached dilatent silty clay loam. The loamy sand to sandy clay loam is brown to dark greyish brown in colour with ped sizes ranging from $<2 \mathrm{~mm}$ up to 5 mm . Ped shapes are granular and polyhedral with ped structure ranging from single grained to weak pedal. The pH of the poorly structured loamy sand to sandy clay loam is highly variable, ranging from 4.5 to 10 with all materials having a high concentrated flow erodibility. The bleached dilatant silty clay loam displays characteristics similar to the other materials comprising the A1 and A2 horizon.

The subsoils of the Jaqua soil landscape unit are of a clayey nature and include a sodic mottled well structured medium clay, a moderately structured clay and a moderately structured dispersive clay. The pH ranges for the moderately structured clay range from 5 to 6 with the ped structure ranging from moderate to strong pedality. Fragment sizes range from fine gravel to coarse gravel with ped shape being sub-angular blocky to angular blocky.

The moderately structured dispersive clay is greyish yellow in colour and has a pH range of 6 to 9 . Fragment sizes are fine gravel to coarse gravel and ped size and shape are consistent with the other materials which comprise the Jaqua sub soil horizon. The concentrated flow erodibility of the dispersive subsoil is high as is the non-concentrated flow erodibility.

## Marulan

The Marulan Soil Landscape unit is found in a small band in southeastern corner of the project area. The upper slopes of the Marulan Soil Landscape Unit contain Paralithic Leptic Rudosols (Lithosols) and Chemic Tenosols (Earthy Sands) with Shallow Red Kandosols (Red Earths) also being present. The mid slopes of the soil landscape unit contain Red Kurosols (Red Podzolic Soils) with the lowers slopes containing Brown Sodosols (Solodic Soils).

The Marulan soil landscape unit typically contains an A1 horizon which consists of a brown coarse sandy loam/sandy clay loam. The fragment sizes range from fine gravel to stones with ped shapes being sub-angular blocky to polyhedral. The structure is massive to weak pedal. The A2 horizon consists of a reddish brown, massive sandy clay loam and a bleached hardsetting sandy loam/clay loam. pH of the A 2 horizon ranges from 5 to 6.5 with fragment sizes ranging from fine gravel to gravel. The concentrated flow erodibility and the non-concentrated flow erodibility potential of both the A1 and A2 horizons is considered to be high.

The subsoil of the Marulan soil landscape unit contains two horizons, the B and B2 horizon. The B horizon is composed of an earthy sandy loam with fragment size ranging from fine gravel to gravel. The pH of the B horizon ranges from 5.5 to 7 with the flow erodibility of the horizon being high.

The B2 horizon consists of red subangular blocky clay and a sodic yellow subangular blocky clay. The red subangular blocky clay has a pH range of 4 to 6.5 , while the sodic yellow subangular blocky clay has a pH range of 5.5 to 7 . Both material are characterised by a high concentrated flow erodibility and a moderate non-concentrated flow erodibility.

